

# **Past Successes of NSF-Sponsored Geotechnical Earthquake Reconnaissance Investigations**

by

**R. B. Seed**

















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# Chang Dam

















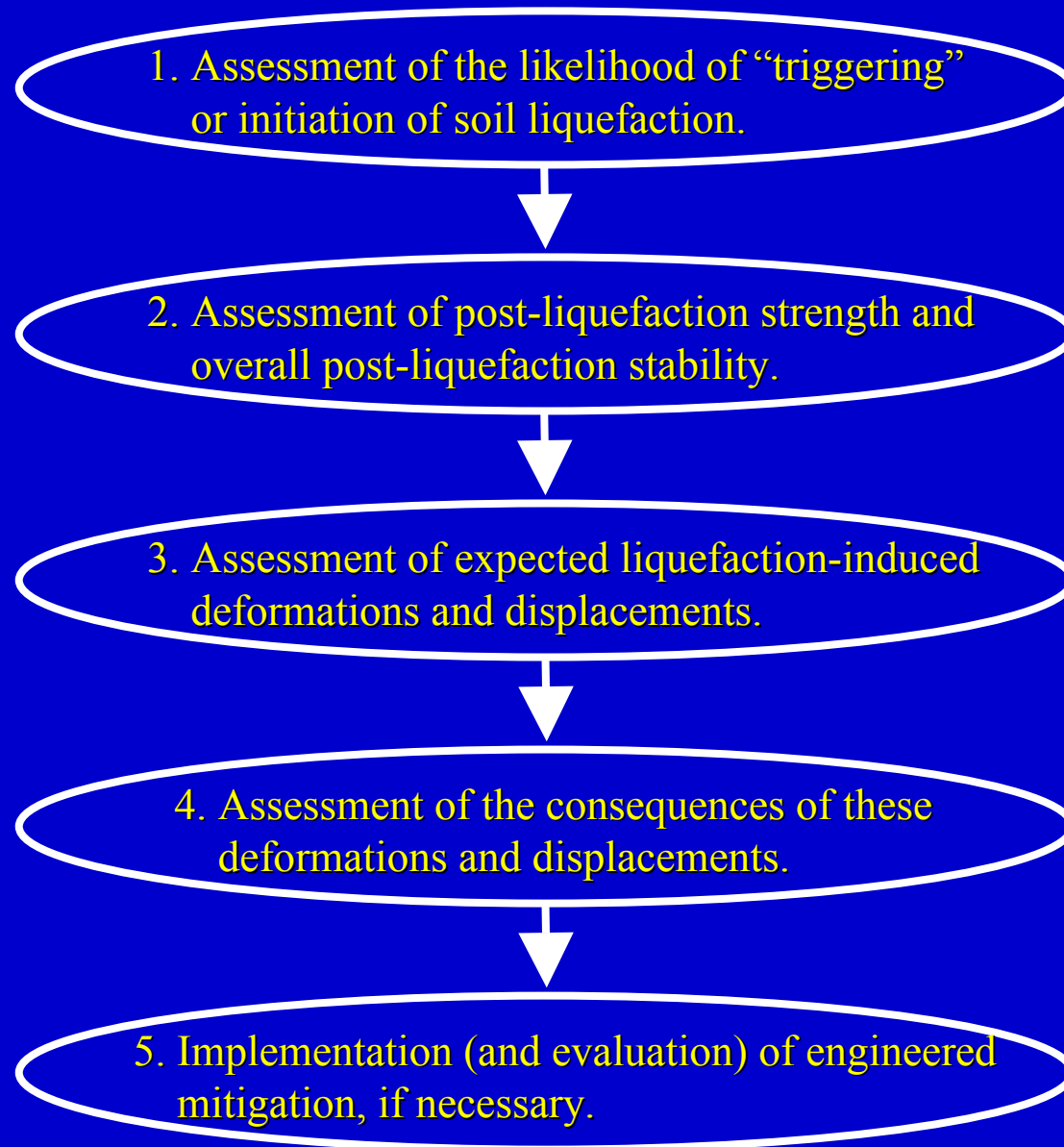




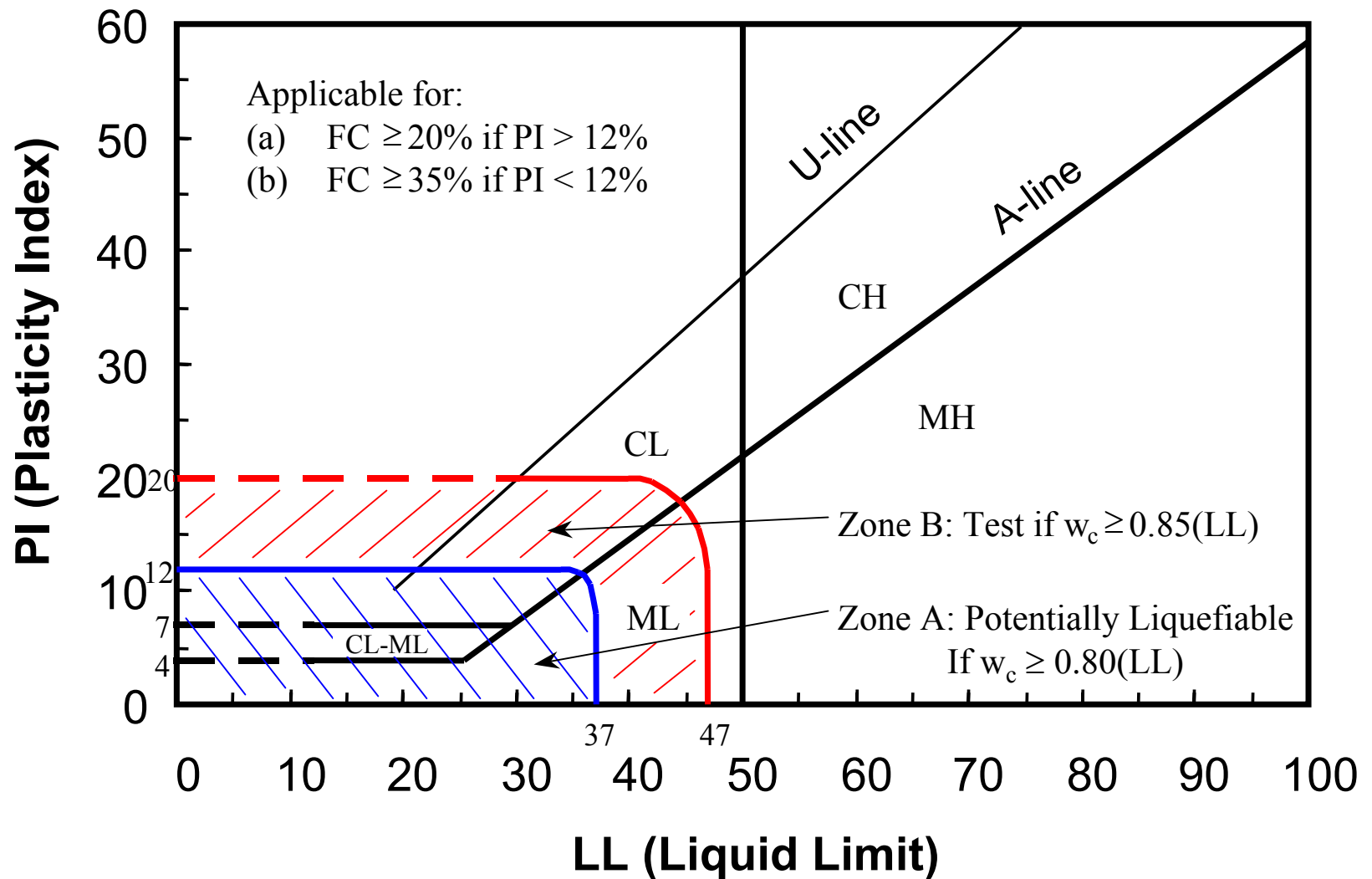




# Key Elements of Soil Liquefaction Engineering







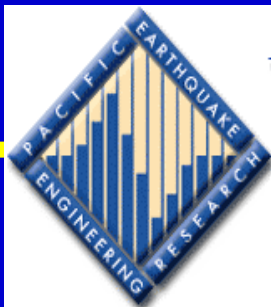
Criteria for Seismically Liquefiable Soils Based on Liquid Limit and Plasticity Index



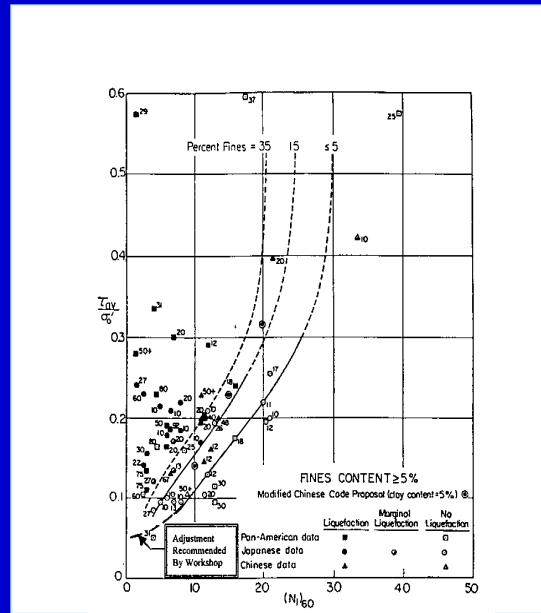
# **SPT-Based Probabilistic Assessment of Soil Liquefaction Triggering Hazard**

by

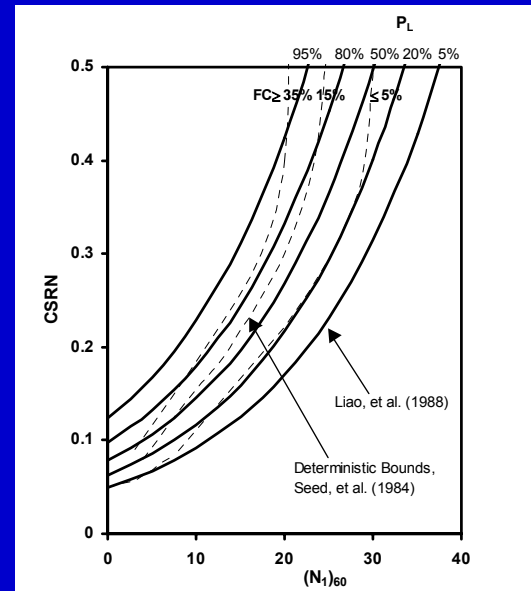
**R. B. Seed, K. O. Cetin, A. Der Kiureghian, R. E. S. Moss,  
K. Tokimatsu, R. E. Kayen, and L. F. Harder, Jr.**



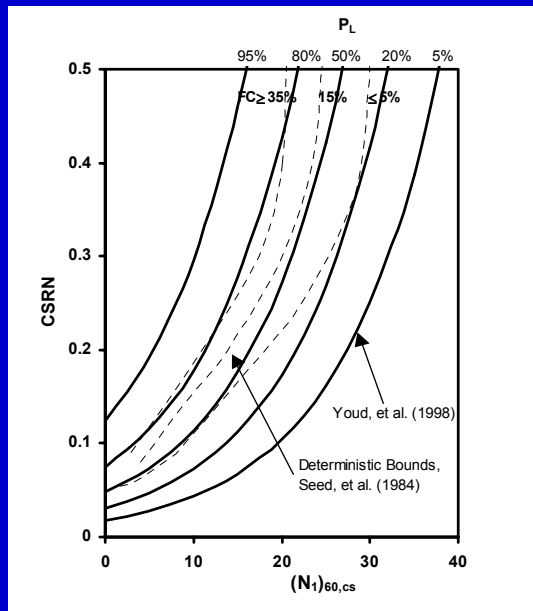
## Seed et al. (1984,85)



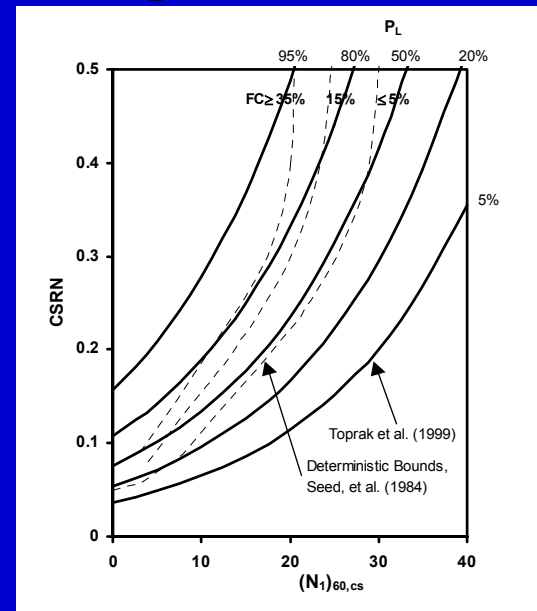
## Liao et al., 1988



## Youd et al., 1998

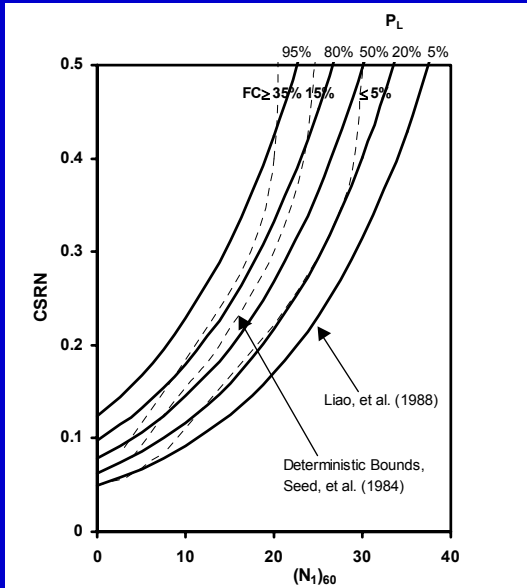


## Toprak et al., 1999

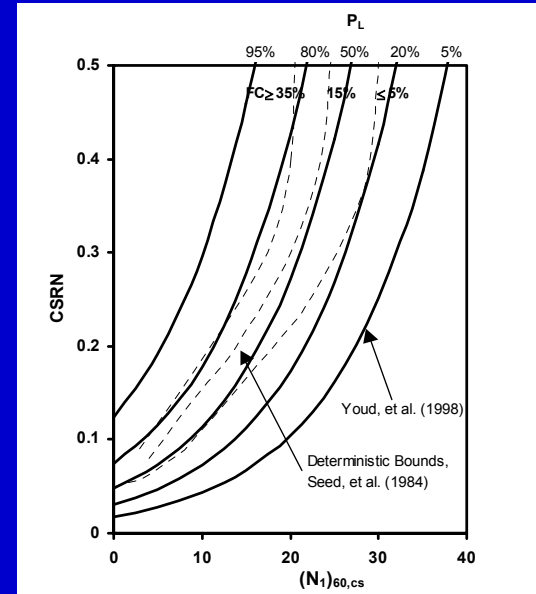




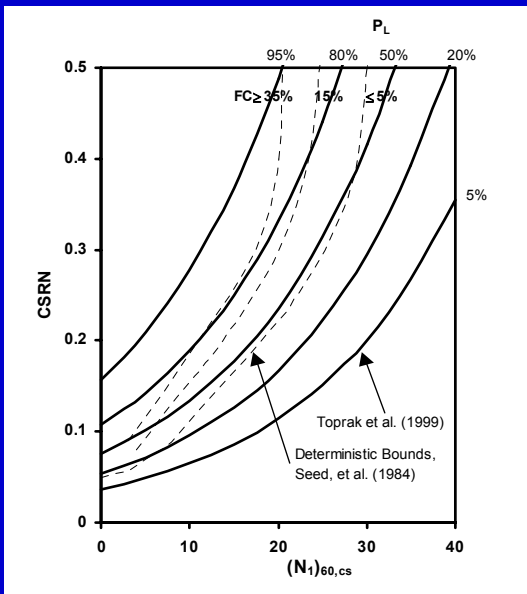
# Liao et al., 1988



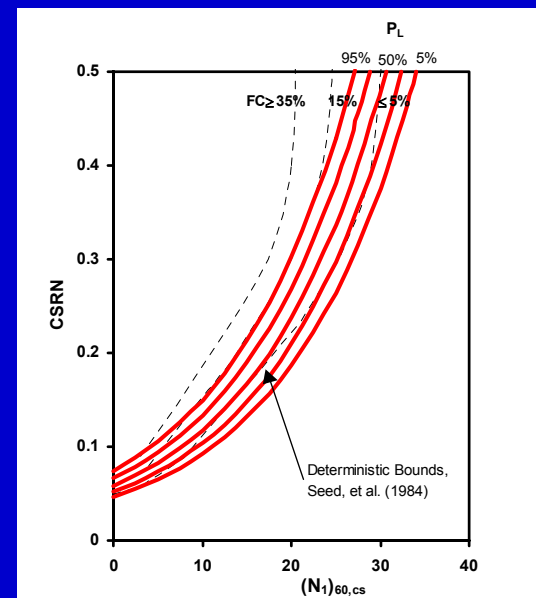
# Youd et al., 1998



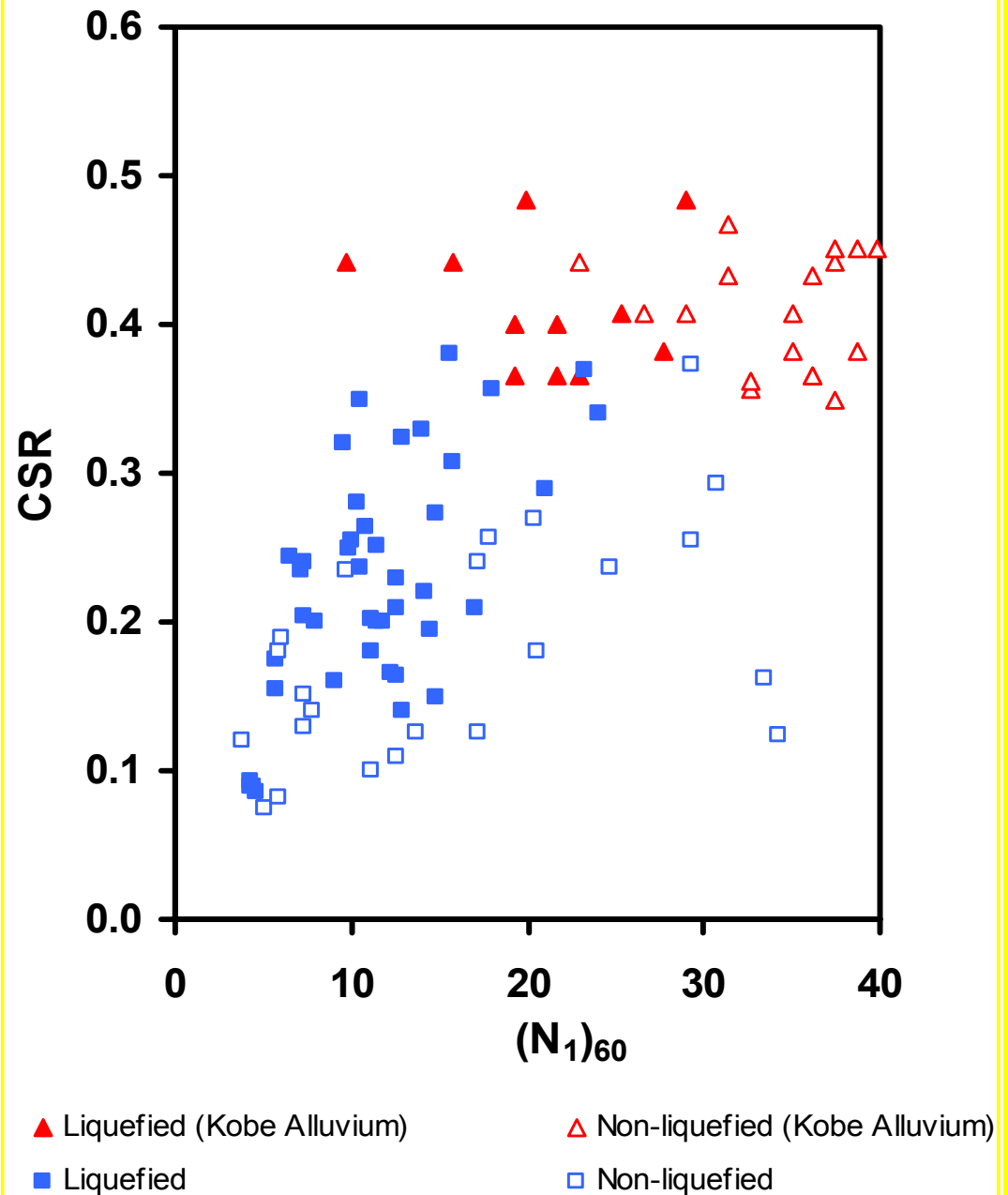
# Toprak et al., 1999



# Cetin et al., 2000



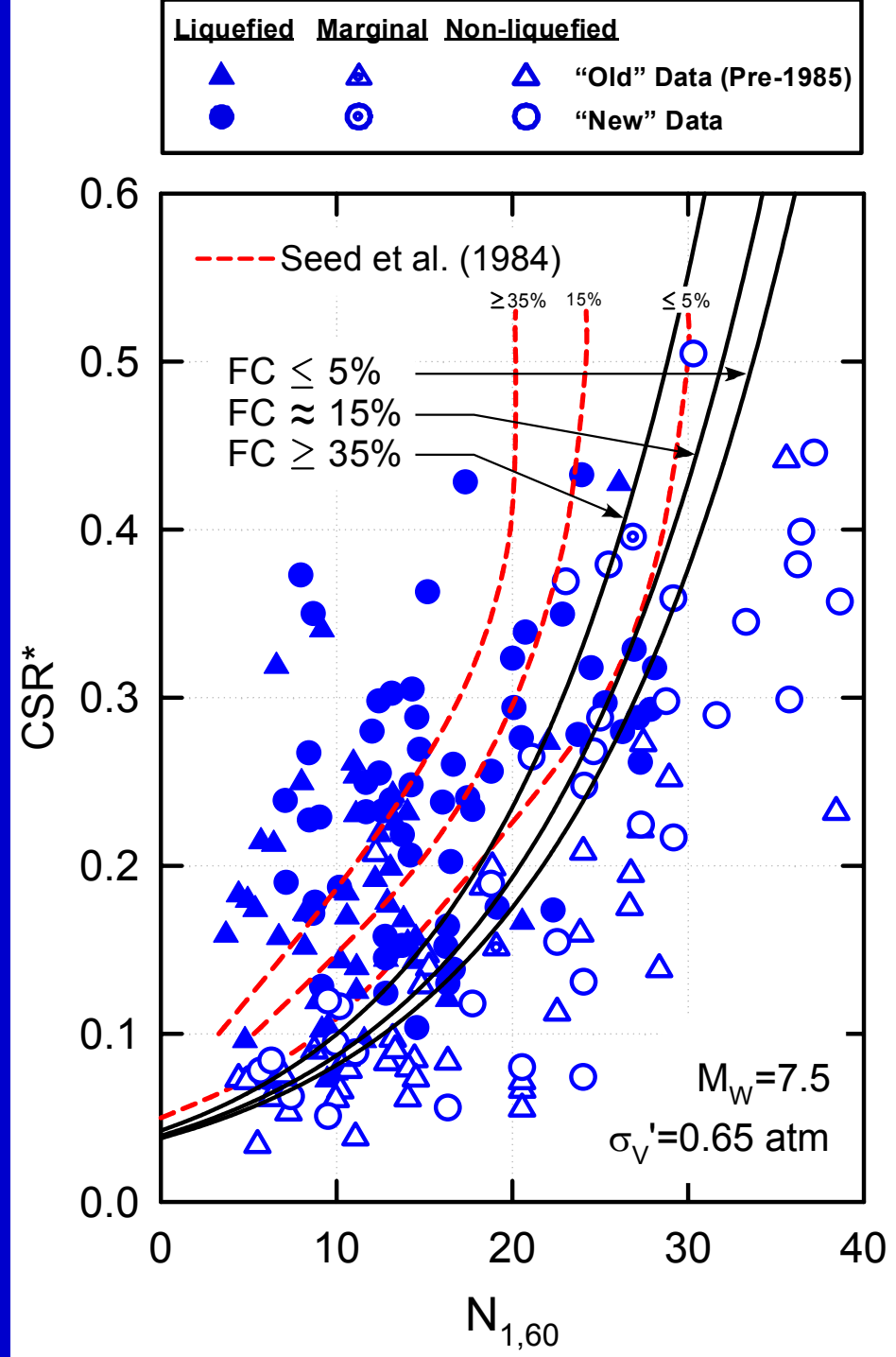
# New Case History Data ( $M_w=6.2-7.8$ )





# Boundary Curves for Probability of Liquefaction = 15%

( $\sigma_v' = 0.65$  atm)



# Magnitude-Correlated Duration Weighting Factors ( $DWF_M$ )

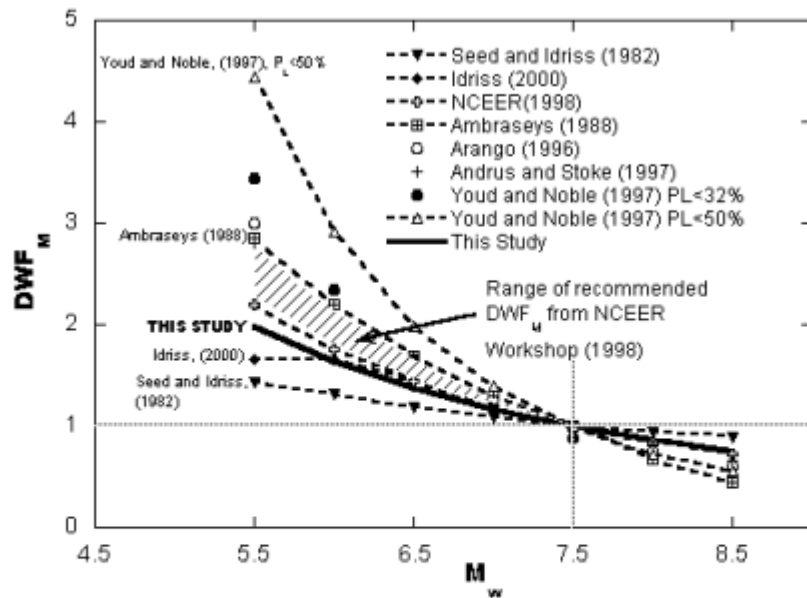


Figure 7(a) : Previous Recommendations for Magnitude-Correlated Duration Weighting Factor, With Recommendations from Current Studies.

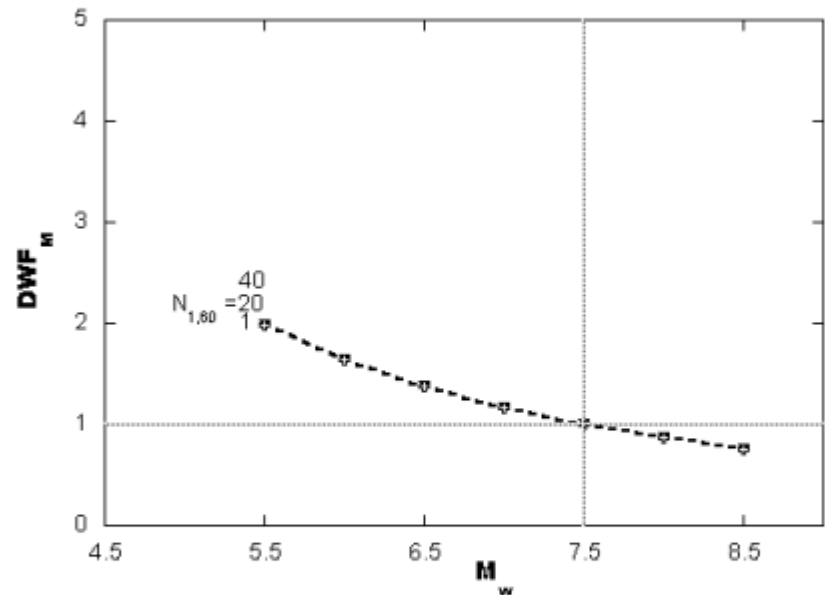


Figure 7(b) : Recommended Magnitude-Correlated Duration Weighting Factor as a Function of  $N_{1,60}$ .



# CPT-Based Probabilistic Assessment of Seismic Soil Liquefaction Initiation

R.E.S. Moss, R.B. Seed, A. Der Kiureghian,  
K.O.Cetin, R.E. Kayen, K. Tokimatsu,  
J.S. Stewart, T.L. Youd and D. Chu



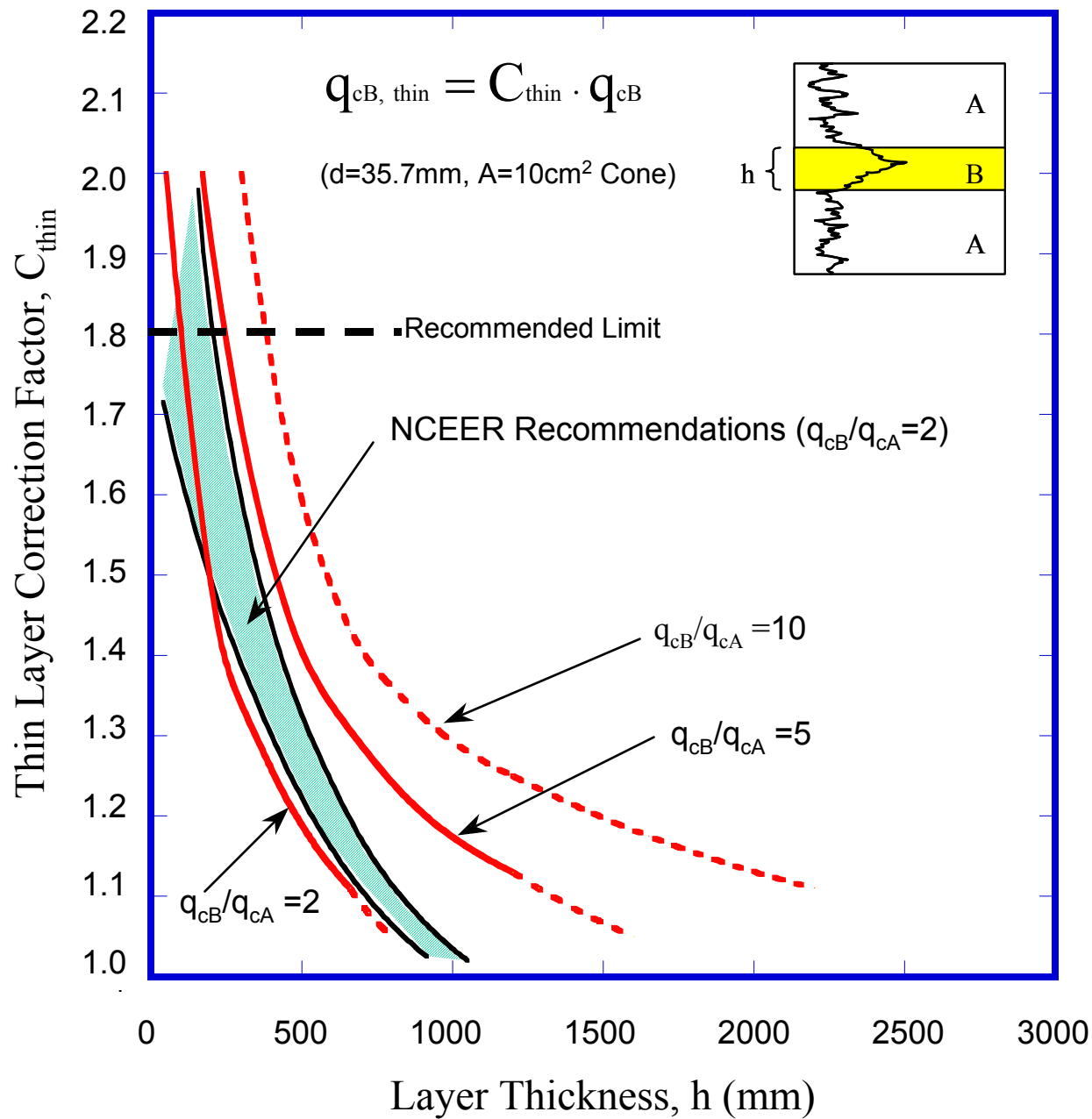
*Pacific Earthquake Engineering  
Research Center (PEER)*

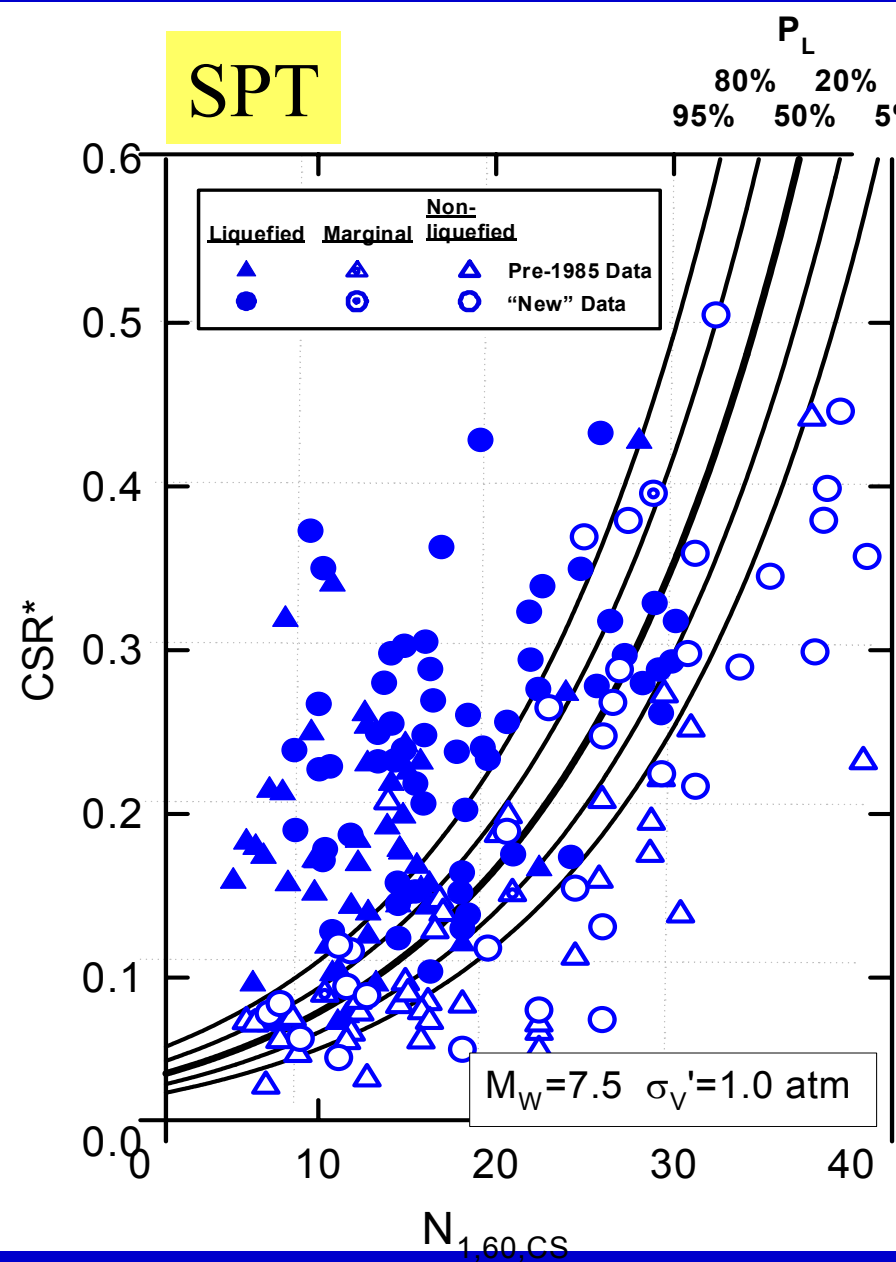
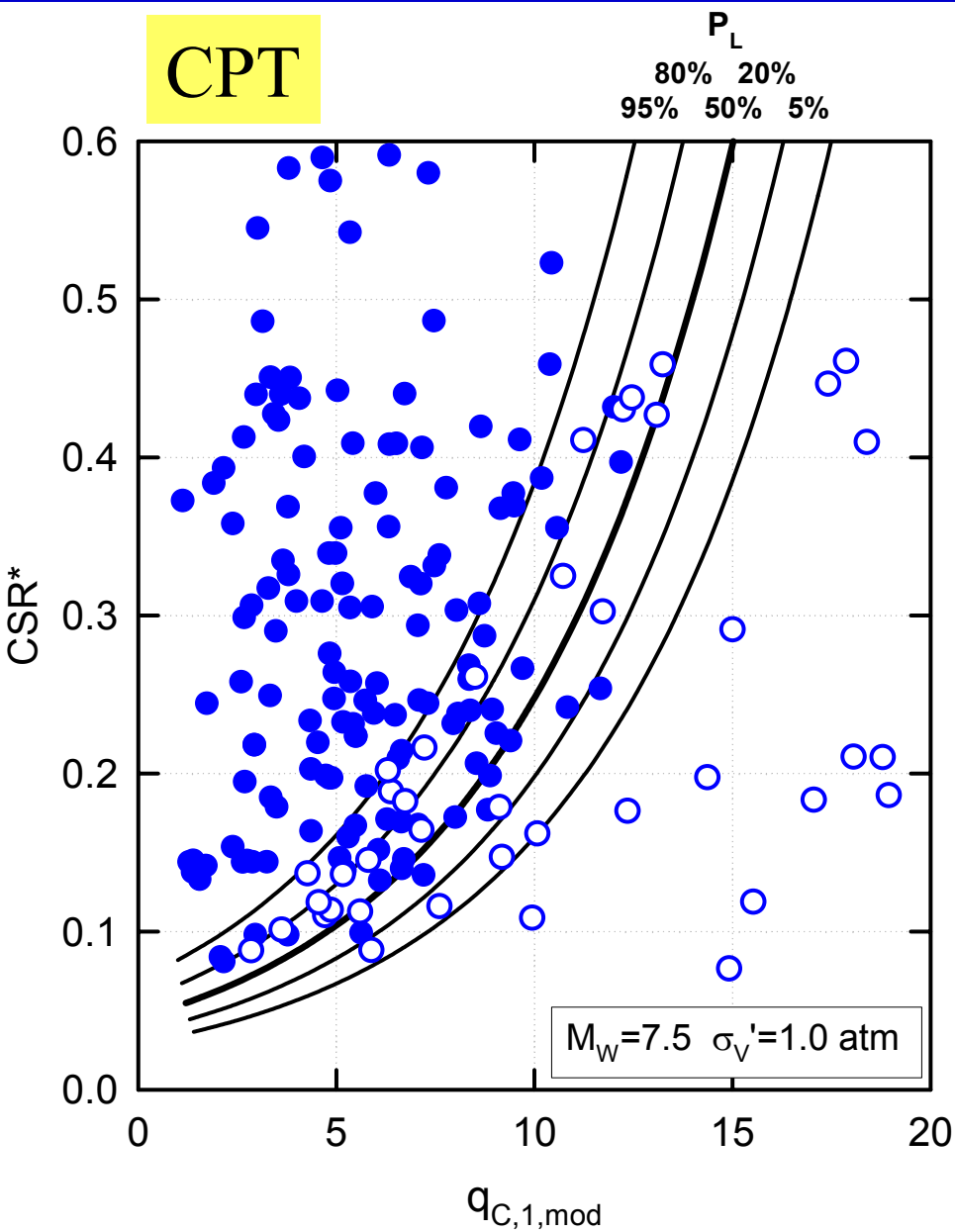
UC Berkeley Caltech Stanford UC Irvine UCLA  
UC Davis UCSD USC University of Washington



| DEPTH<br>(feet) | DEPTH<br>(meters) | CONE PENETRATION TEST |                          | SPT - N | GRAIN SIZE           |           | CLASS    | UNIT | GRAPHIC LOG | SOIL DESCRIPTION                                                                                                                                                                                                                                                                                                              |
|-----------------|-------------------|-----------------------|--------------------------|---------|----------------------|-----------|----------|------|-------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
|                 |                   | RATIO<br>%            | TIP<br>MN/m <sup>2</sup> |         | D <sub>50</sub> (mm) | FINES (%) |          |      |             |                                                                                                                                                                                                                                                                                                                               |
| 0               | 0.0               |                       |                          |         |                      |           |          |      |             |                                                                                                                                                                                                                                                                                                                               |
| 5               | 1.5               |                       |                          |         |                      |           |          |      |             | Fill                                                                                                                                                                                                                                                                                                                          |
| 5               | 1.5               |                       |                          |         |                      |           |          |      |             | Silty SAND                                                                                                                                                                                                                                                                                                                    |
| 10              | 3.0               |                       |                          | 10      | 0.105<br>0.072       | 37<br>51  | SM<br>ML |      |             | Silty SAND to sandy SILT, dark yellowish brown 10YR4/6, loose, very friable, not sticky or plastic, sharp contact between the two sediment types, modern root, no gravel                                                                                                                                                      |
| 15              | 4.5               |                       |                          |         |                      |           |          |      |             | Lean CLAY to sandy lean CLAY                                                                                                                                                                                                                                                                                                  |
| 20              | 6.0               |                       |                          |         |                      |           |          |      |             | Silty SAND                                                                                                                                                                                                                                                                                                                    |
| 20              | 6.0               |                       |                          |         | 0.037<br>0.072       | 65<br>51  | CL       |      |             | Sandy lean CLAY, brown to dark brown 10YR4/3, uniform, soft, massive, slightly sticky and plastic, pebbles to 15 mm                                                                                                                                                                                                           |
| 20              | 6.0               |                       |                          |         |                      |           |          |      |             | Lean CLAY with sand to sandy SILT, dark brown 10YR3/3, massive, soft and loose, effervescent.                                                                                                                                                                                                                                 |
| 30              | 9.0               |                       |                          |         | 0.048<br>0.011       | 64<br>82  | ML<br>CL |      |             | Silty SAND to sandy SILT, dark brown-brown 10YR4/3 to dark yellowish brown 10YR4/6, massive, well defined upward fining sequence continues into sample above                                                                                                                                                                  |
| 35              | 10.5              |                       |                          |         | 0.015<br>0.255       | 77<br>29  | CL<br>SM |      |             | Sandy SILT to lean CLAY with sand, upper part is dark yellowish brown 10YR4/6 the lower part is heavily mottled; light brownish gray, very dark grayish brown, Lean CLAY with sand to silty SAND with gravel, dark grayish brown to olive brown 2.5Y4/3 and dark yellowish brown 10YR4/3, massive, this sample is mostly clay |
| 40              | 12.0              |                       |                          | 17      | 0.085<br>0.038       | 46<br>65  | SM<br>CL |      |             | Silty SAND and sandy lean CLAY, dark grayish brown to olive brown 2.5Y4/3, some olive yellow mottling, slight effervescence, friable                                                                                                                                                                                          |





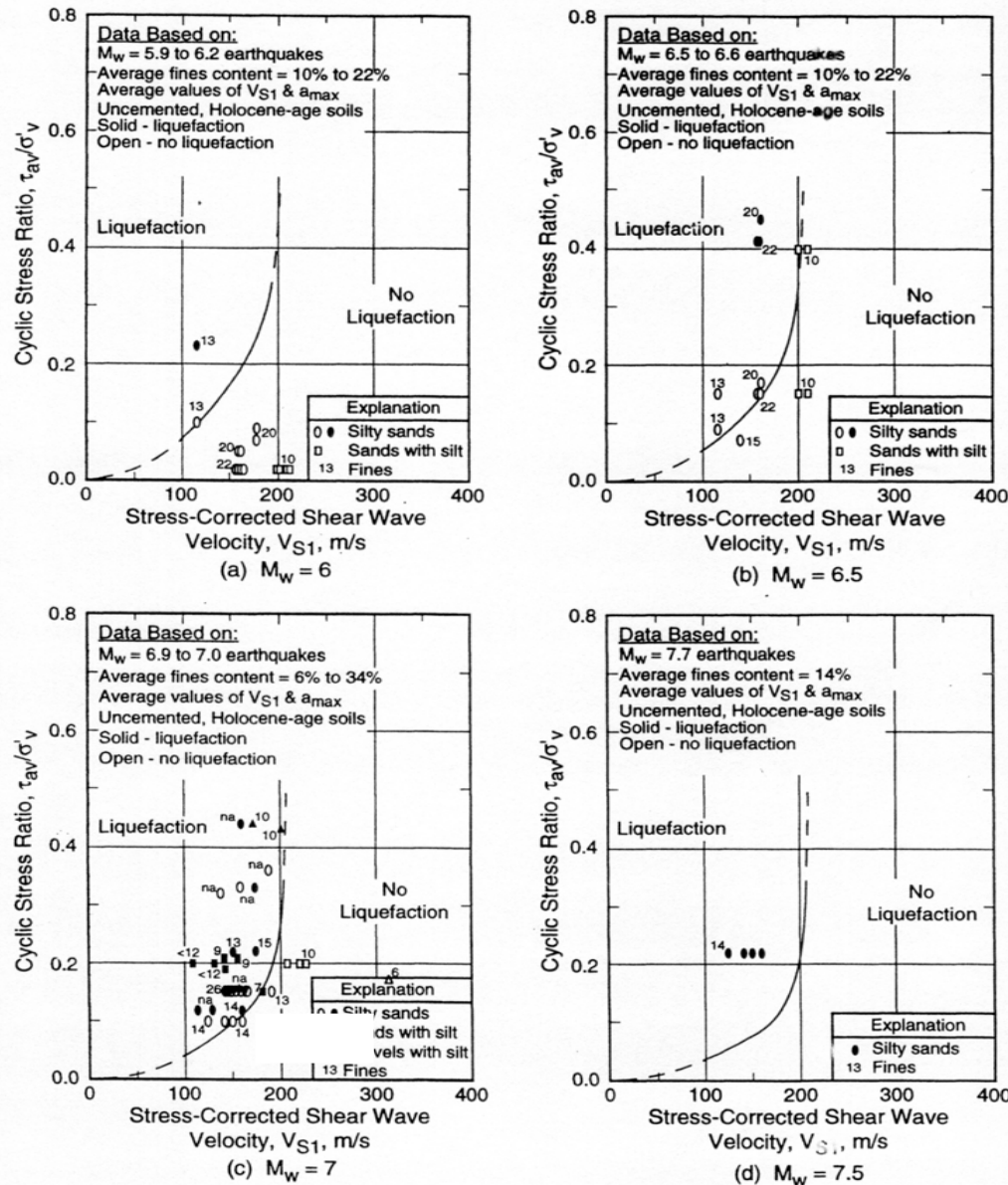


Equivalent "Clean Sand"



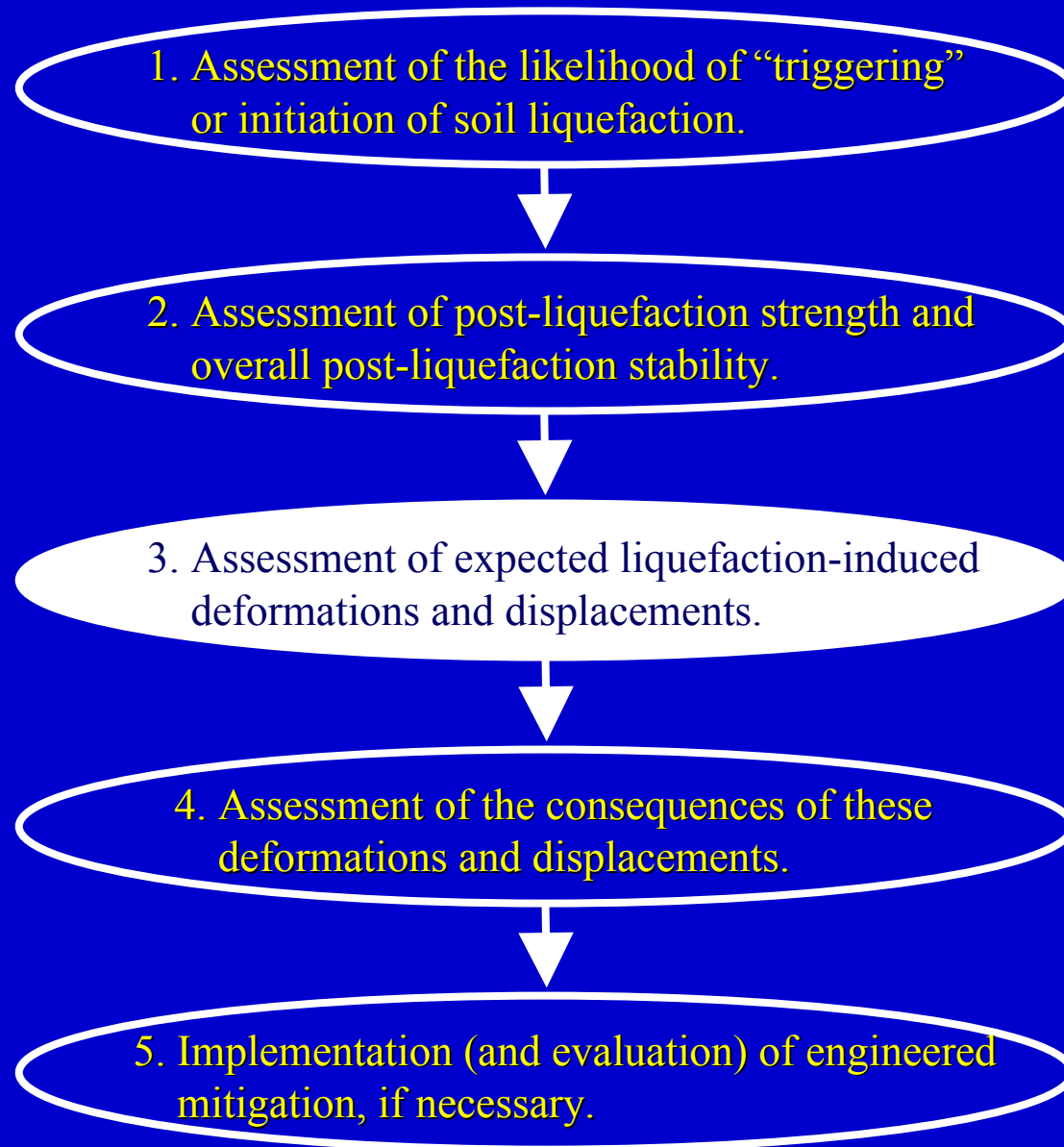
# V<sub>S</sub>

## Andrus & Stokoe (1997)



**Fig. 9 - Comparison of Liquefaction Assessment Charts Based on  $V_{S1}$  and CSR from Analysis for this Report with Case Histories of Uncemented Soils with Fines Content of 6% to 34%.**

# Key Elements of Soil Liquefaction Engineering





# Liquefaction Induced Deformations Under Uni-Directional and Multi-Directional Loading

**R. B. Seed, Annie Kammerer, Jiaer Wu,  
J. M. Pestana, M. F. Riemer,  
L. Salvati, J. Mayoral, P. Meymand**



*Pacific Earthquake Engineering  
Research Center (PEER)*

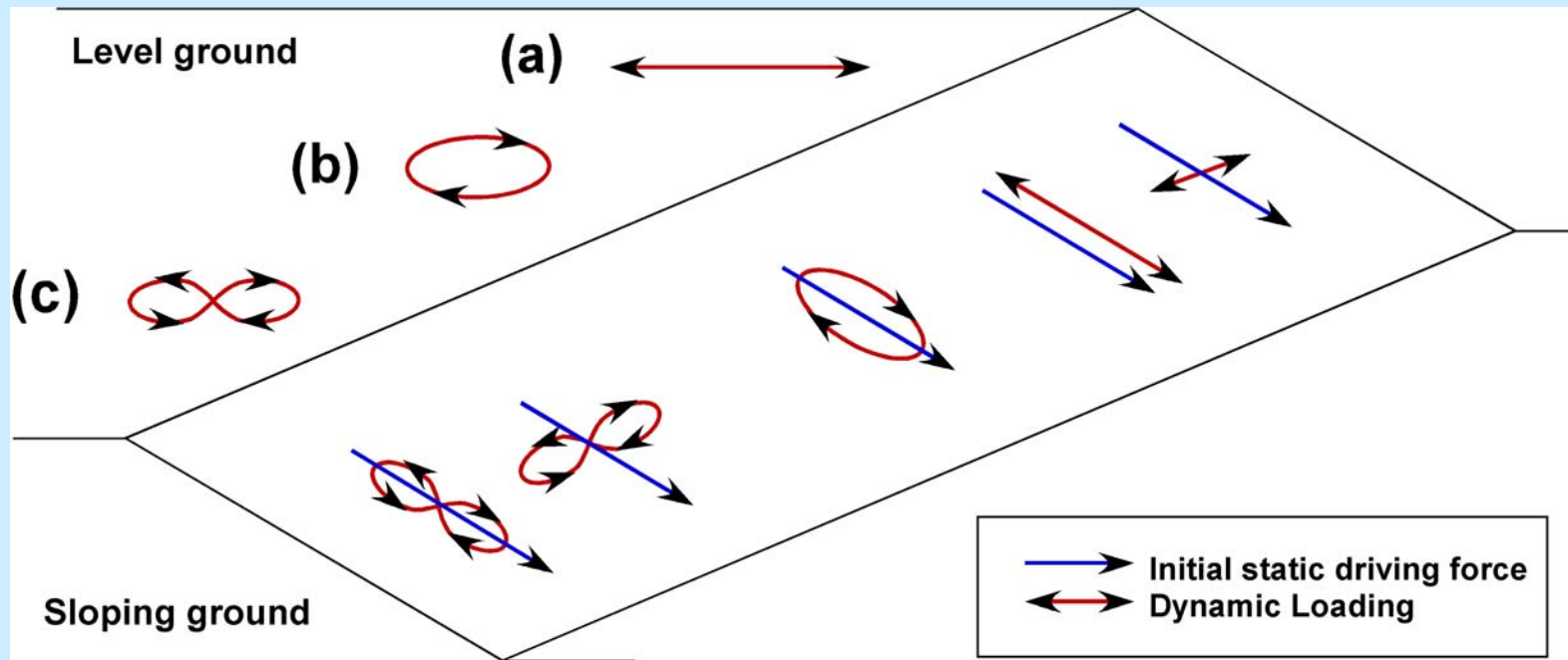
UC Berkeley Caltech Stanford UC Irvine UCLA  
UC Davis UCSD USC University of Washington



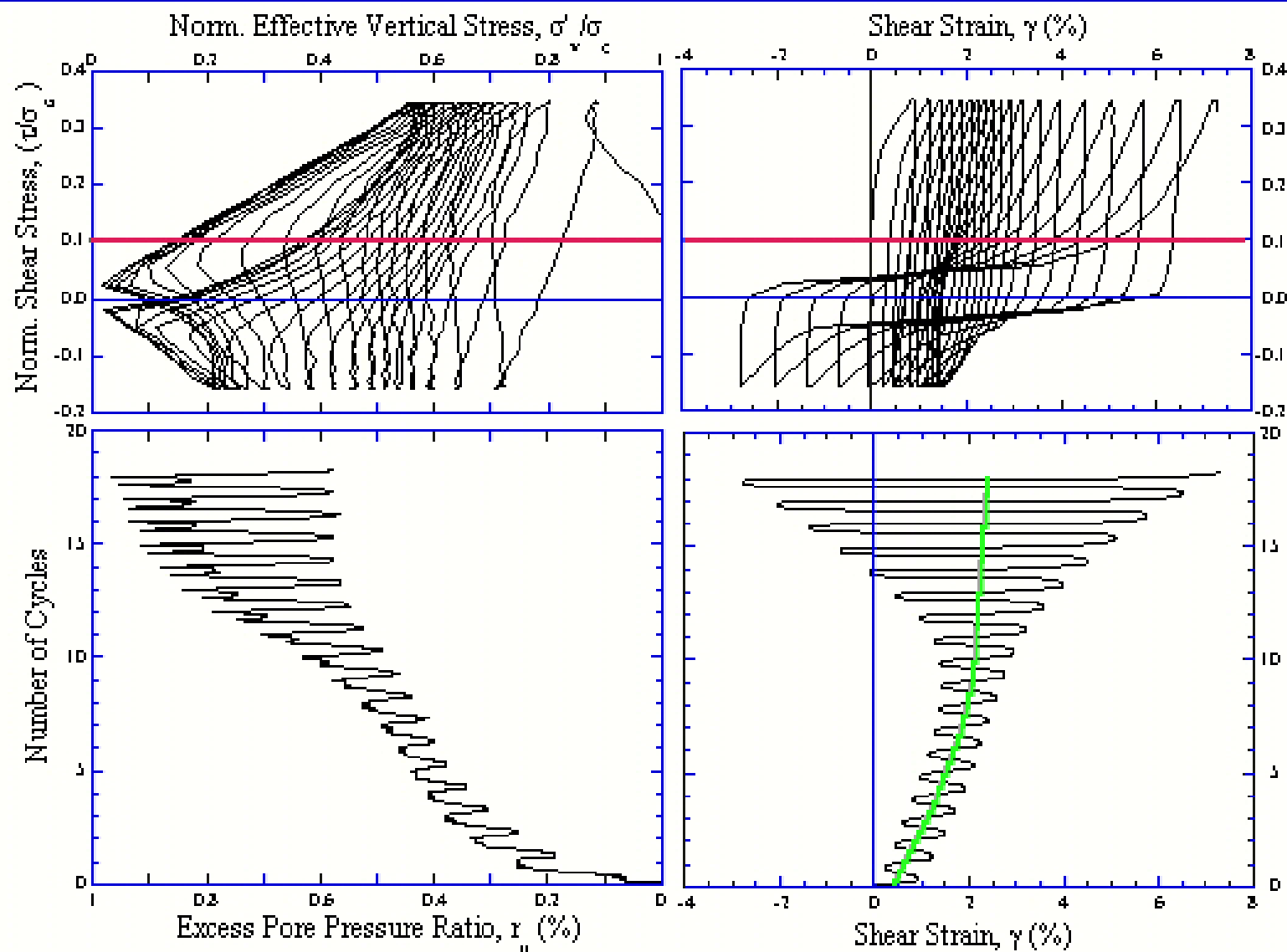




# Summary of Testing



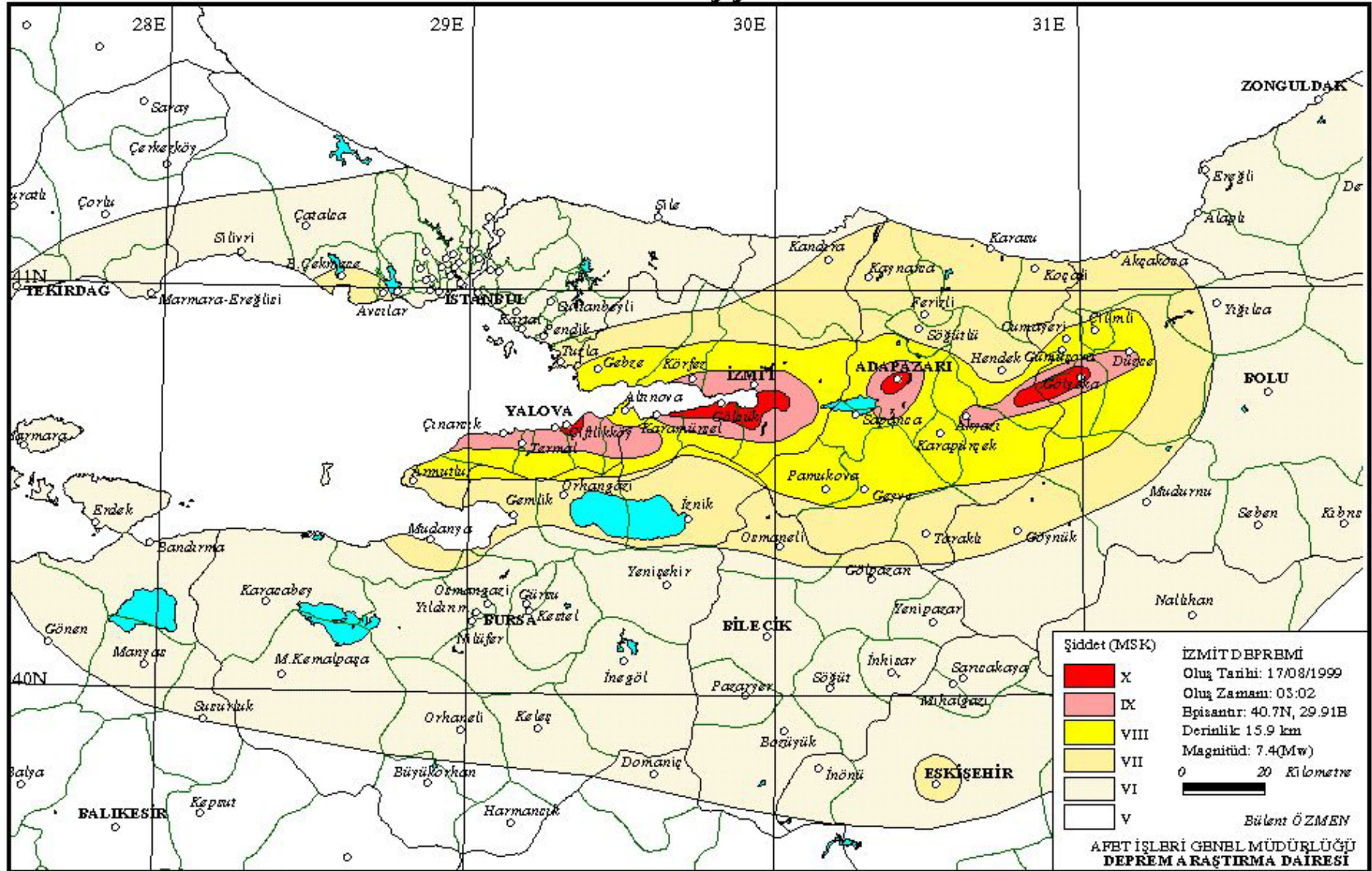
**Schematic Illustration of 2-Directional Loading Conditions**



$D_r=65\%$   $\sigma_v=40$  kPa CRR= 0.25  $\alpha= 0.10$



# İZMİT DEPREMİNİN EŞİDDET HARİTASI



Şekil 3 : İzmit depreminin eşiddet haritası

Isoseismal Map from the August 17, 1999 Kocaeli, Turkey Earthquake ( $M_w = 7.4$ ) ([http://www.deprem.gov.tr/main\\_e.htm](http://www.deprem.gov.tr/main_e.htm))













# GÜRKAN RESTAURANT

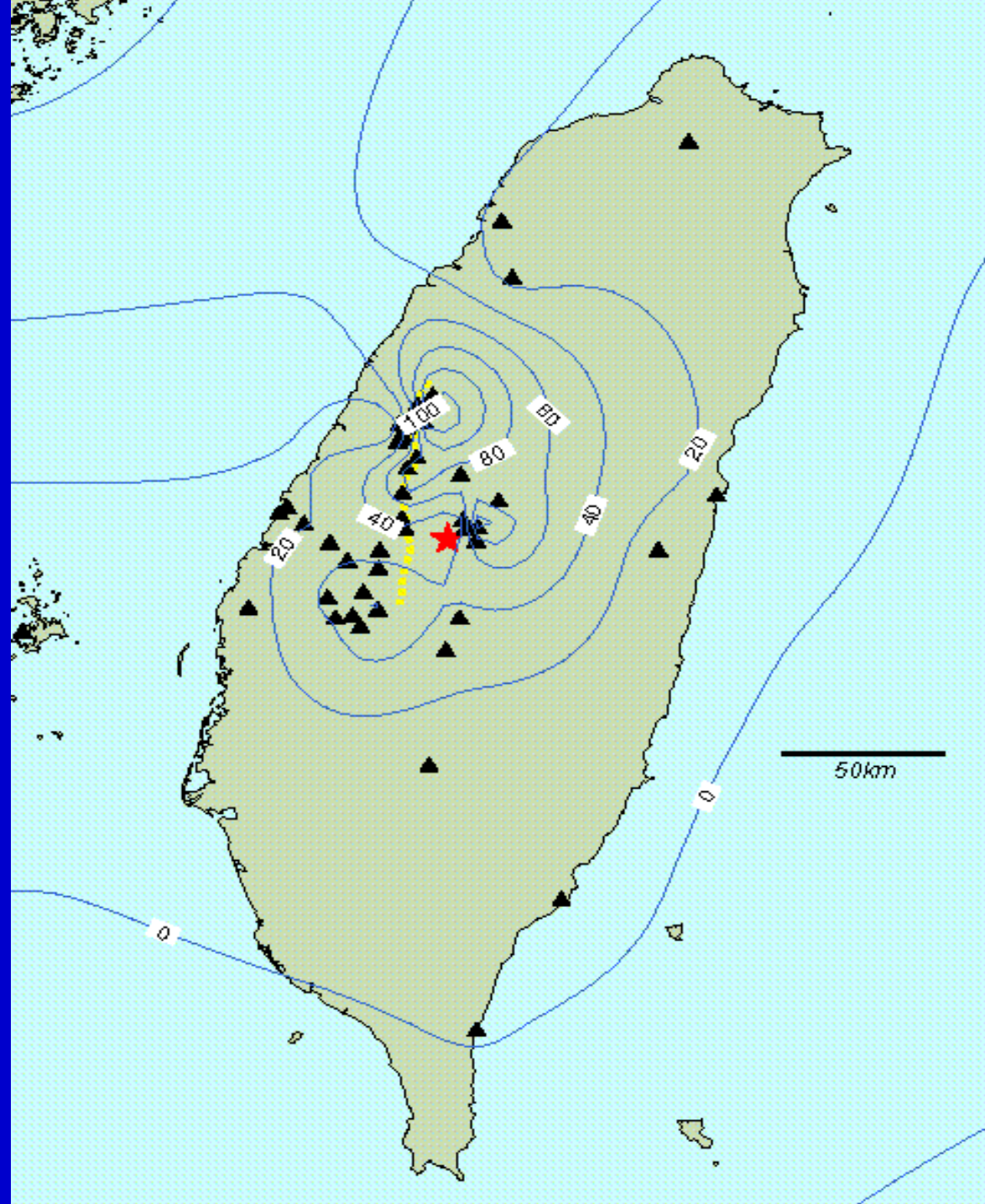


# Adapazari

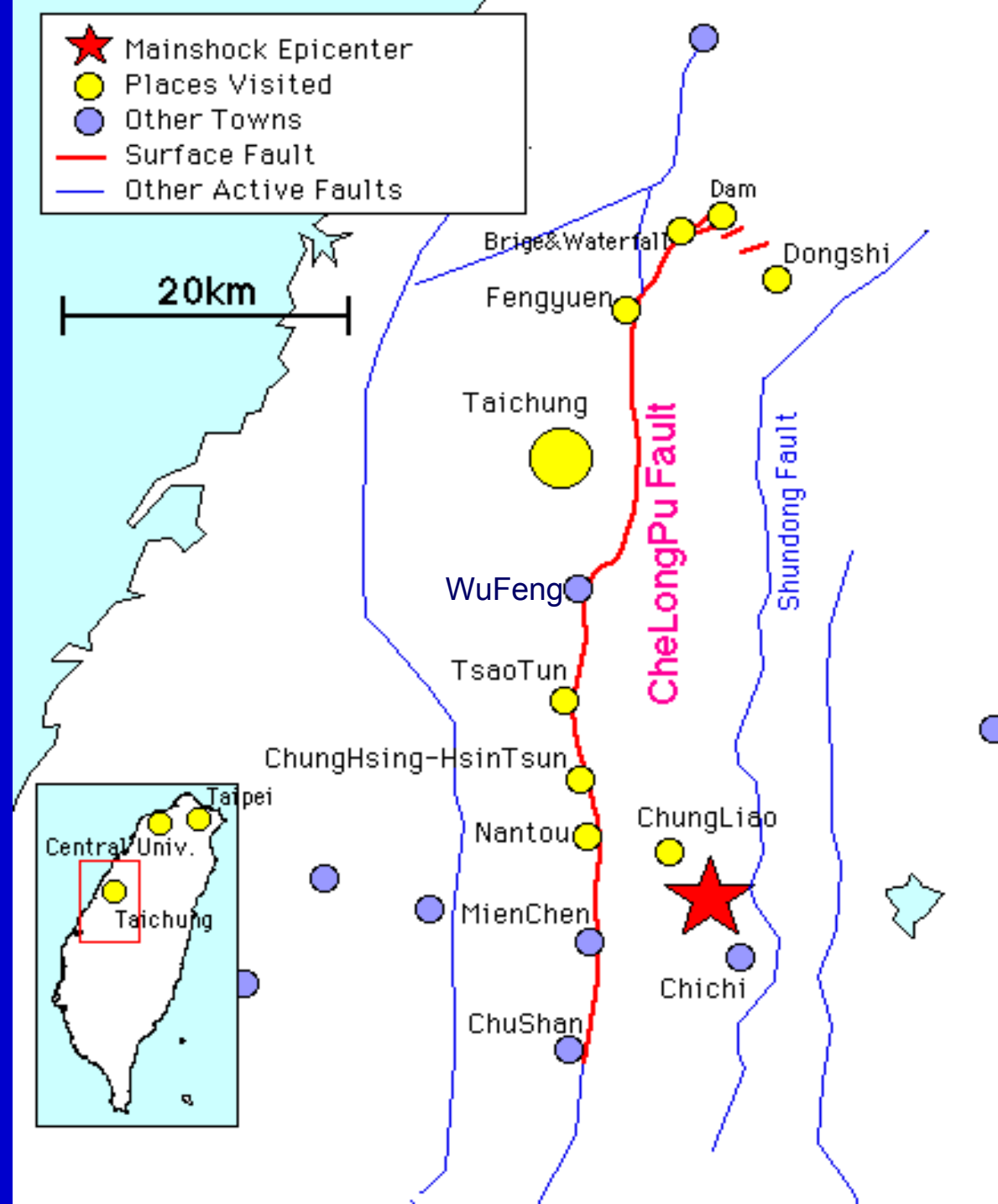


(O. Cetin, 1999)





- ★ Mainshock Epicenter
- Places Visited
- Other Towns
- Surface Fault
- Other Active Faults







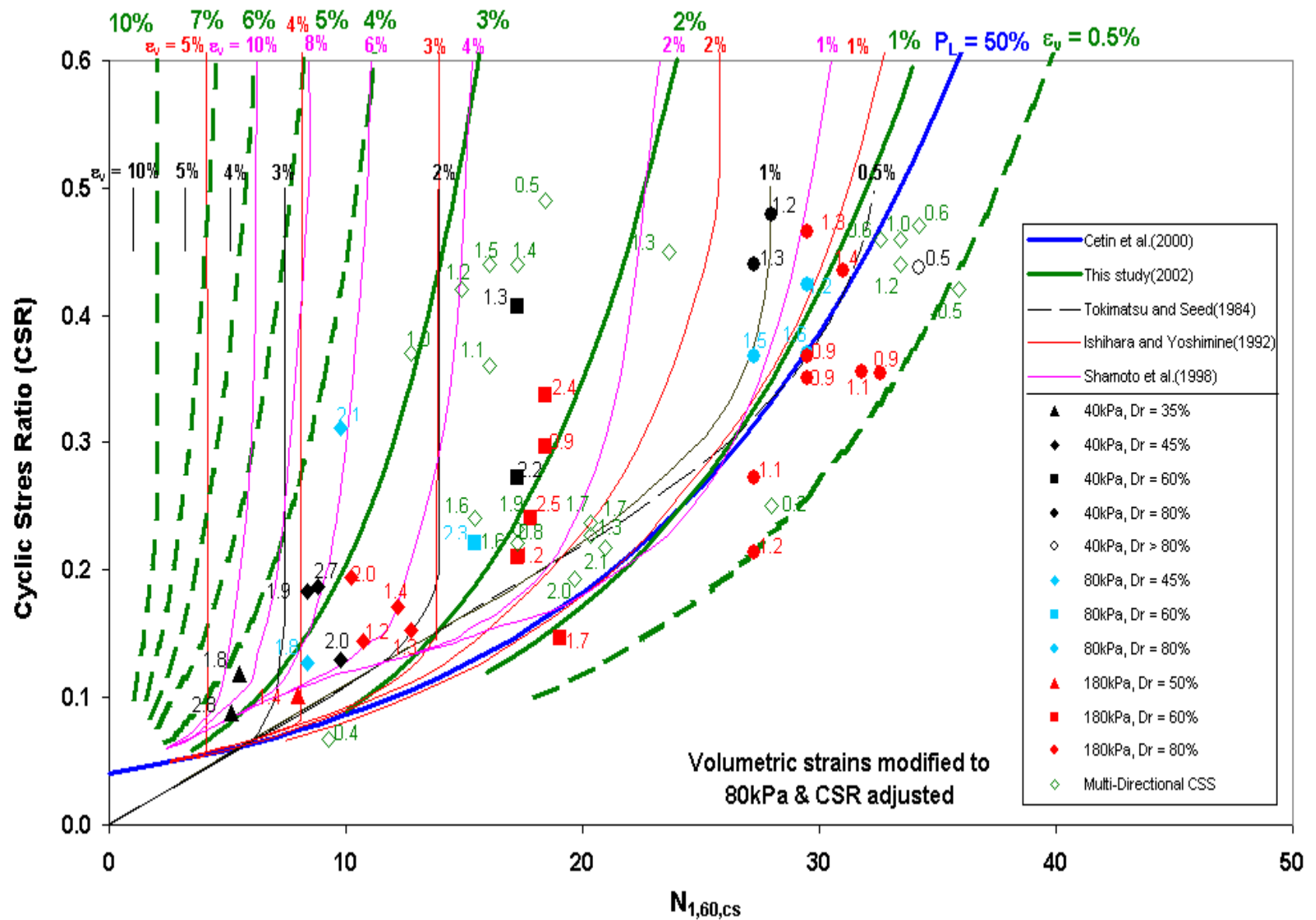




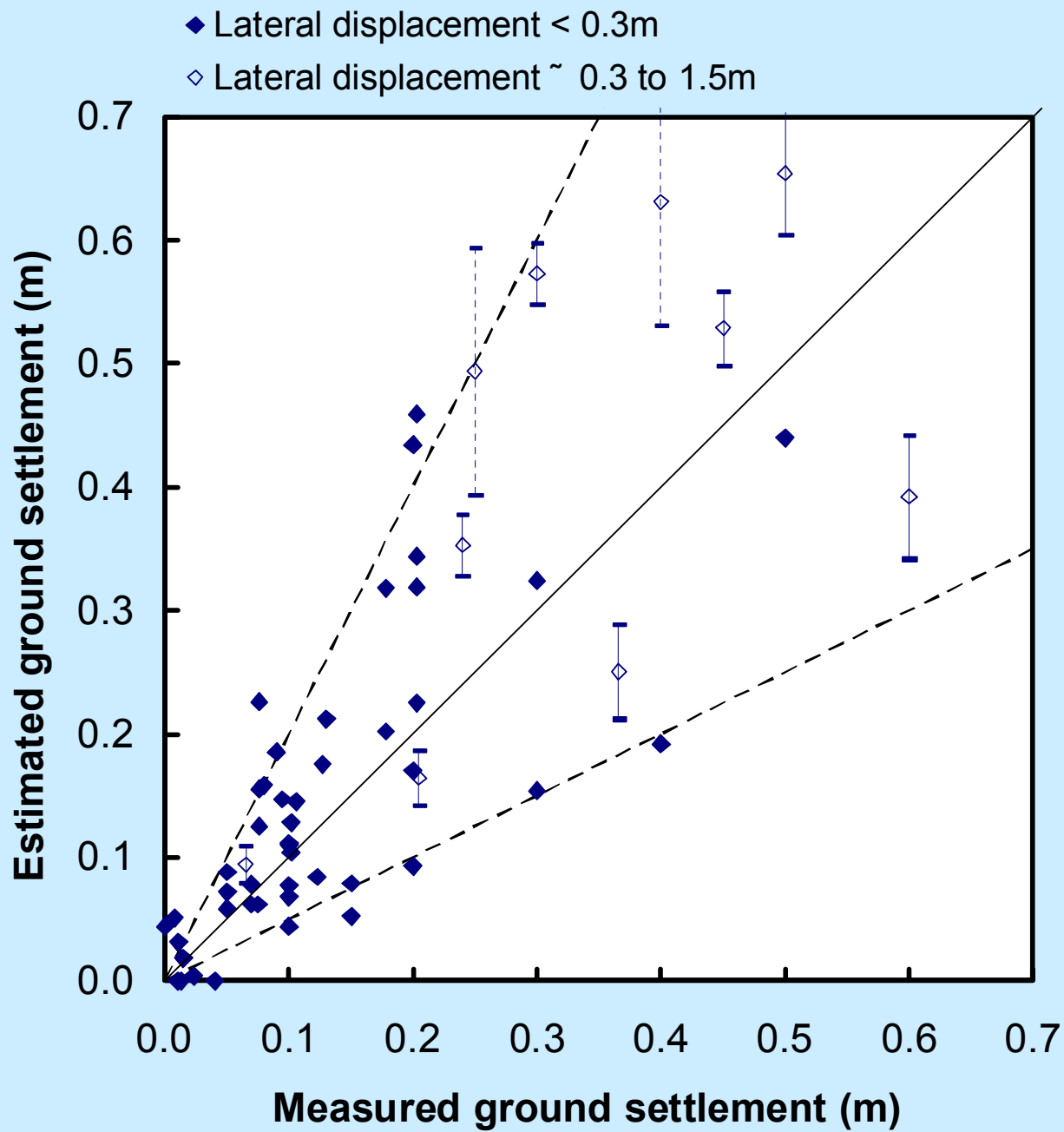


# Post-Liquefaction Reconsolidation Ground Settlements







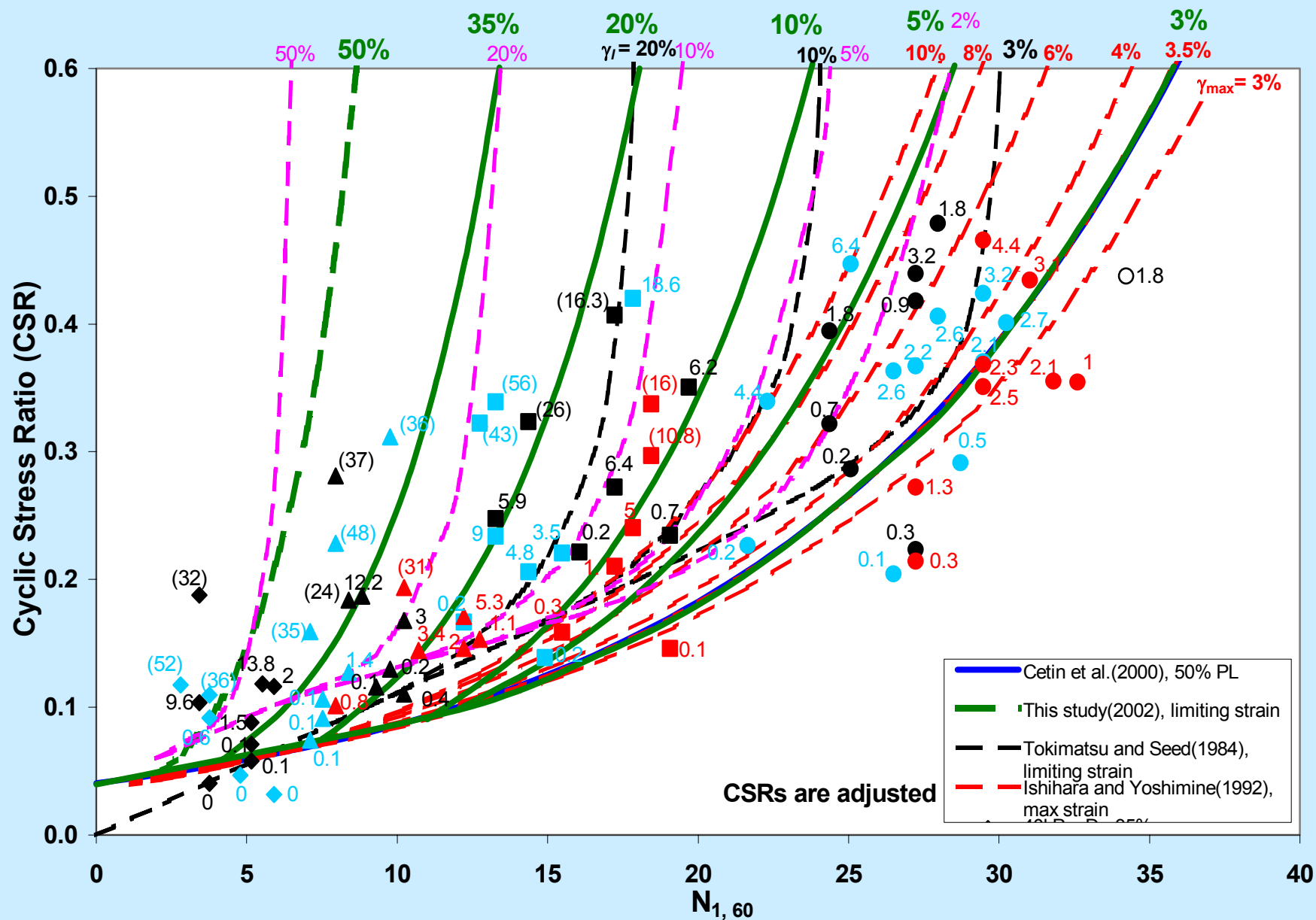


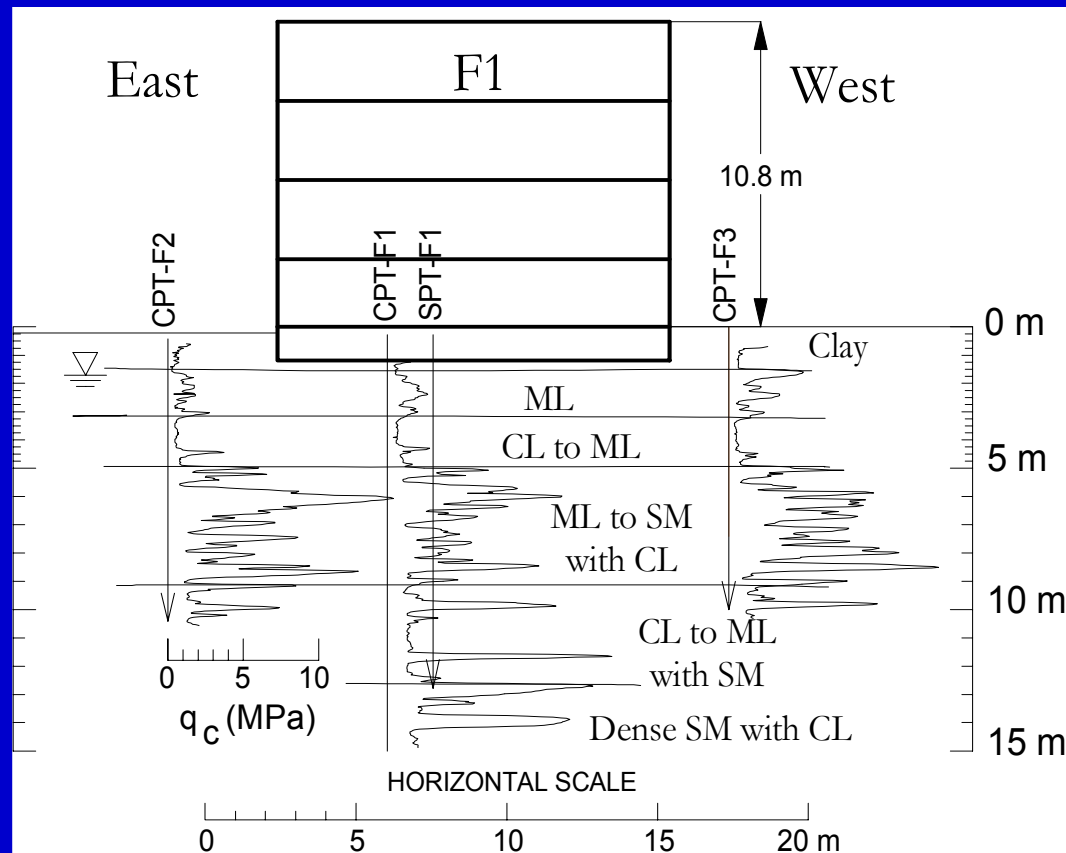
# Liquefaction-Induced Building Settlements





## Relationship between Cyclic Stress Ratio, N1-values and Shear Strains



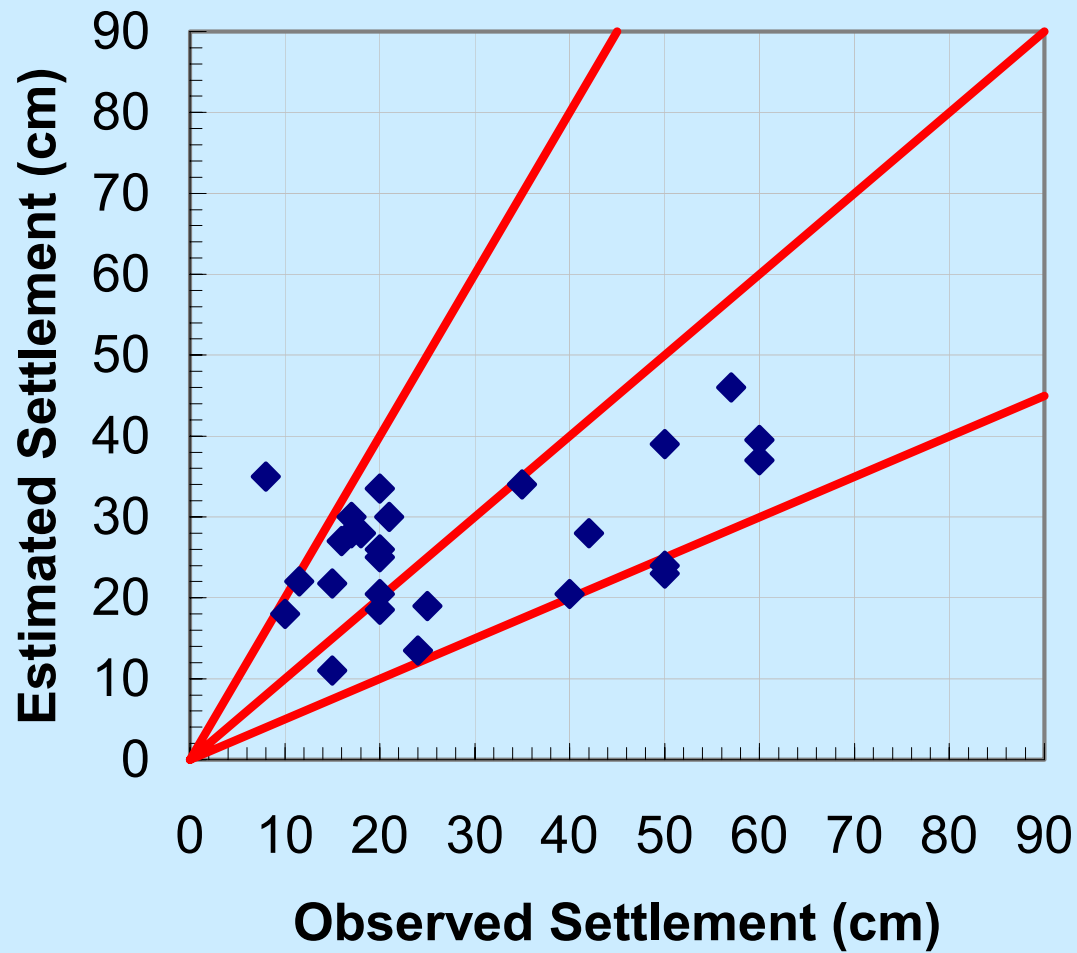


$$\Delta Z_{\text{total}} = \Delta Z_{\text{volumetric}} + \Delta Z_{\text{deviatoric}}$$

$$\Delta Z_{\text{deviatoric}} = f(\text{CSR}_{\text{free-field}}, \text{CSR}_{\text{SSI}}, \text{SPI}, \alpha, K_{\alpha, \varepsilon})$$



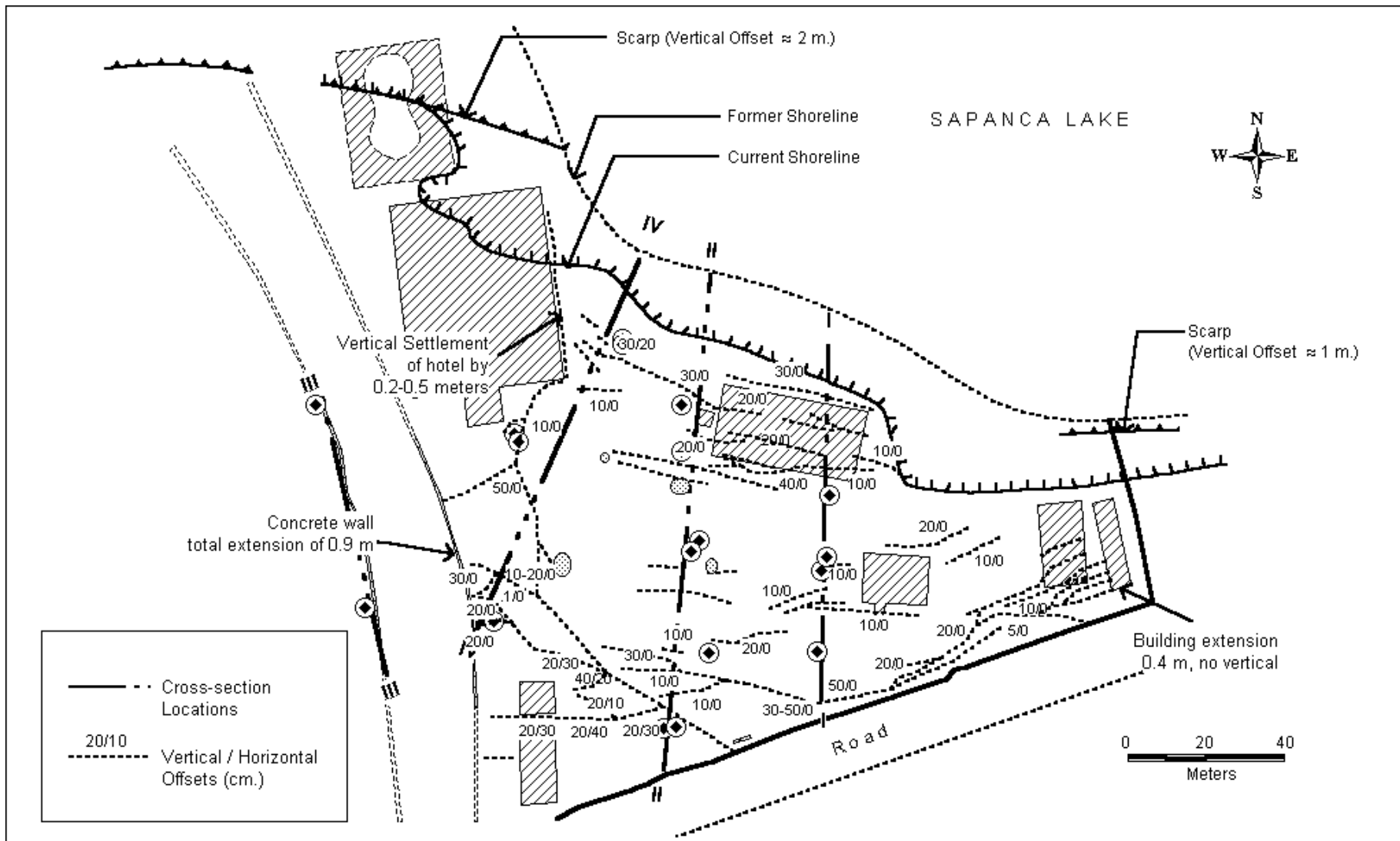
## Total Building Settlement



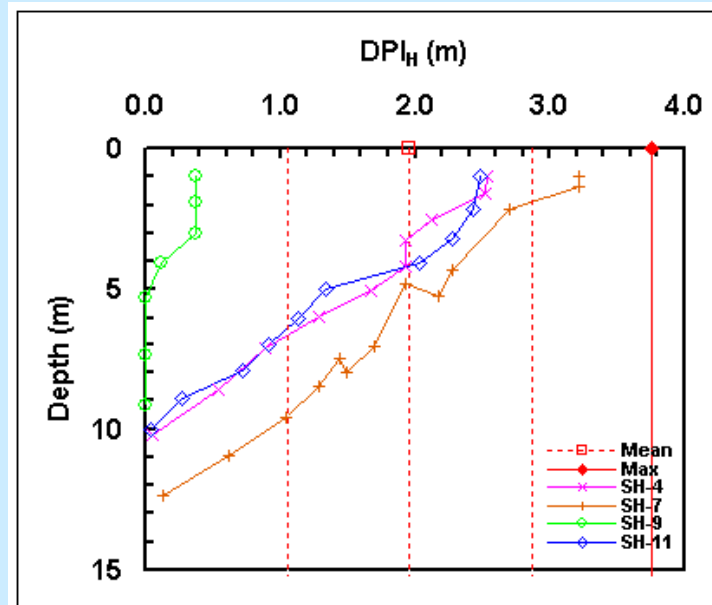
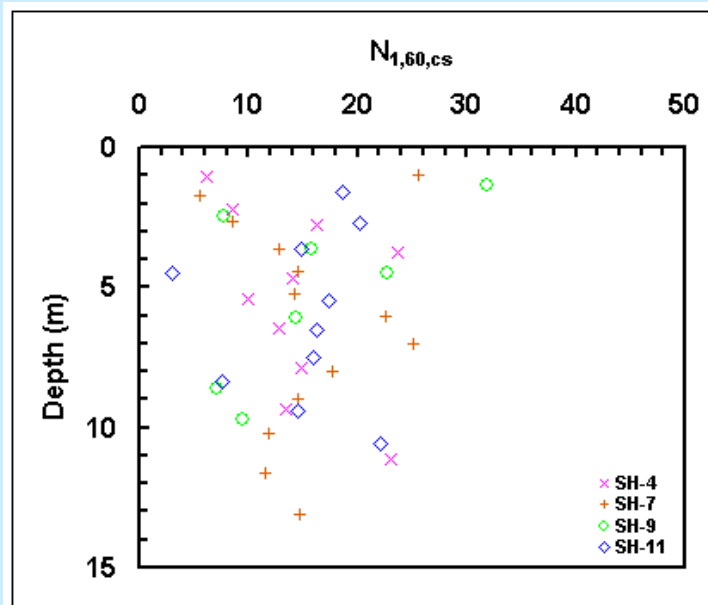
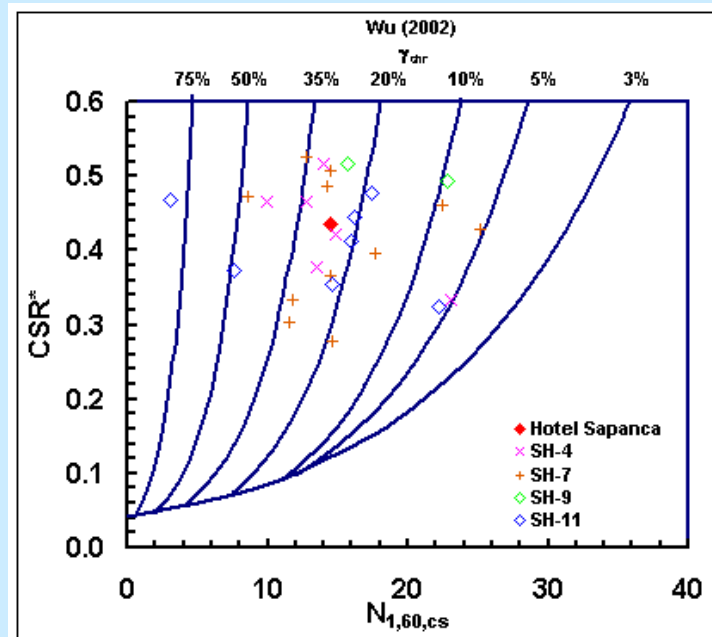
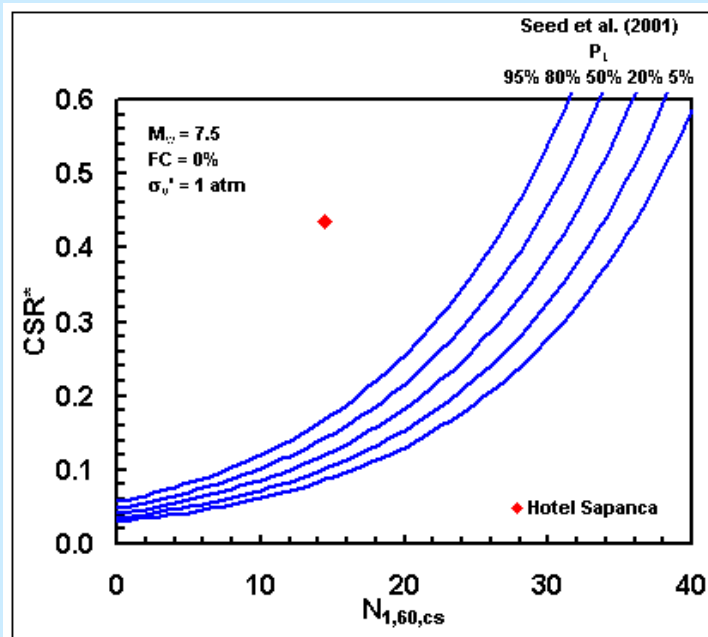
# Hotel Sapanca, 1999 Kocaeli Earthquake



# Hotel Sapanca, 1999 Kocaeli Earthquake

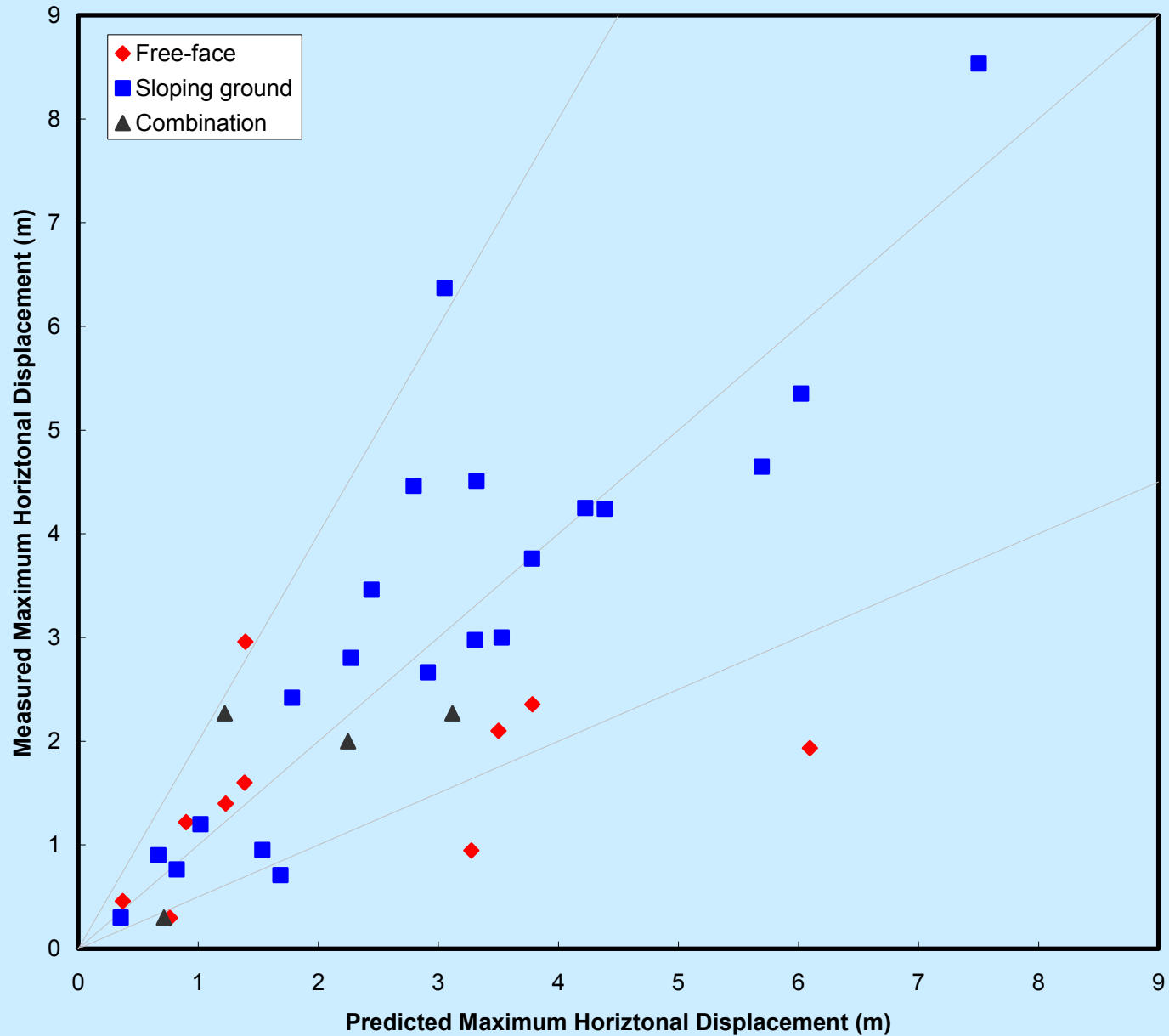




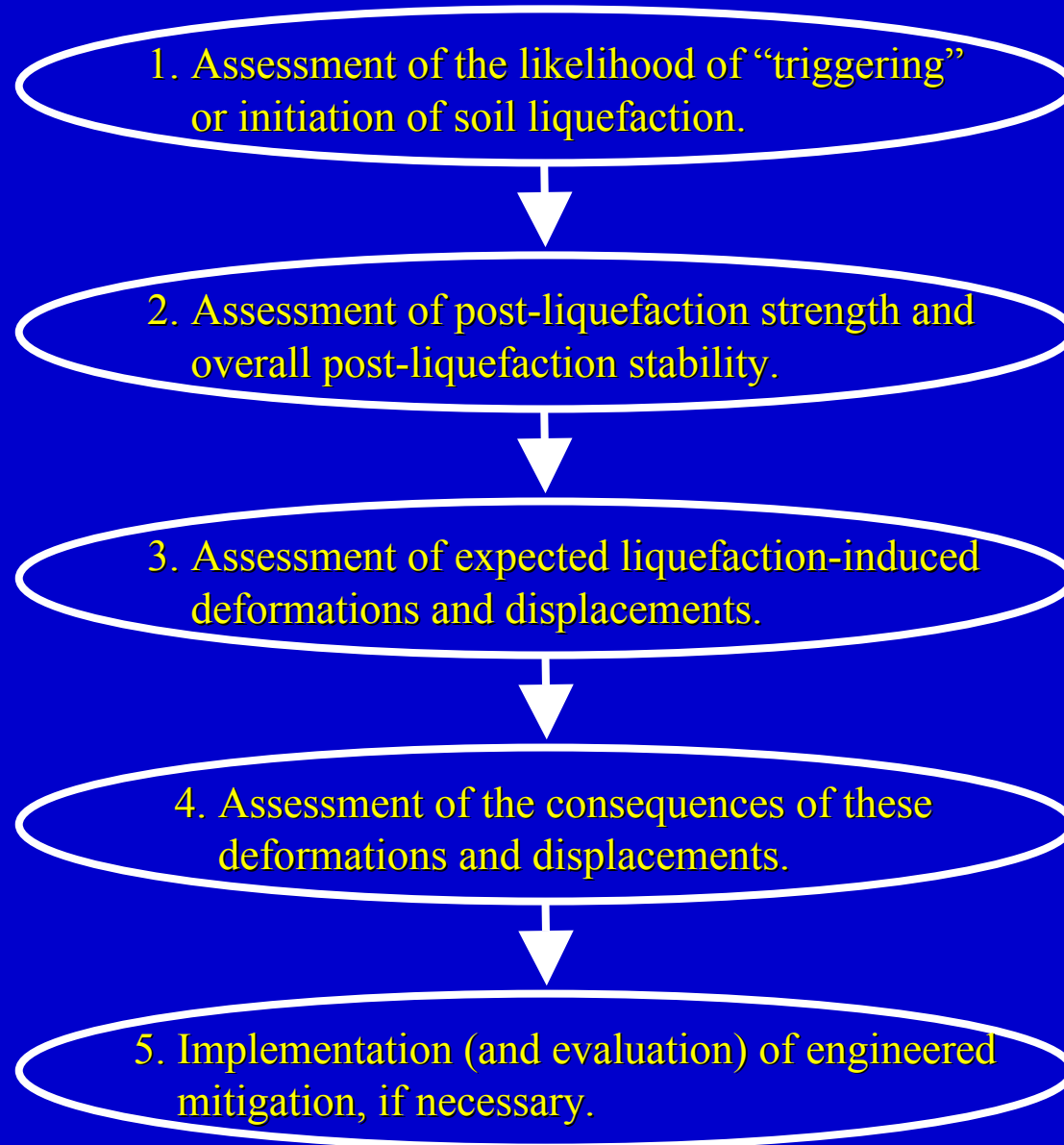


Plots of Liquefaction-Induced Lateral Spreading Assessment for Hotel Sapanca

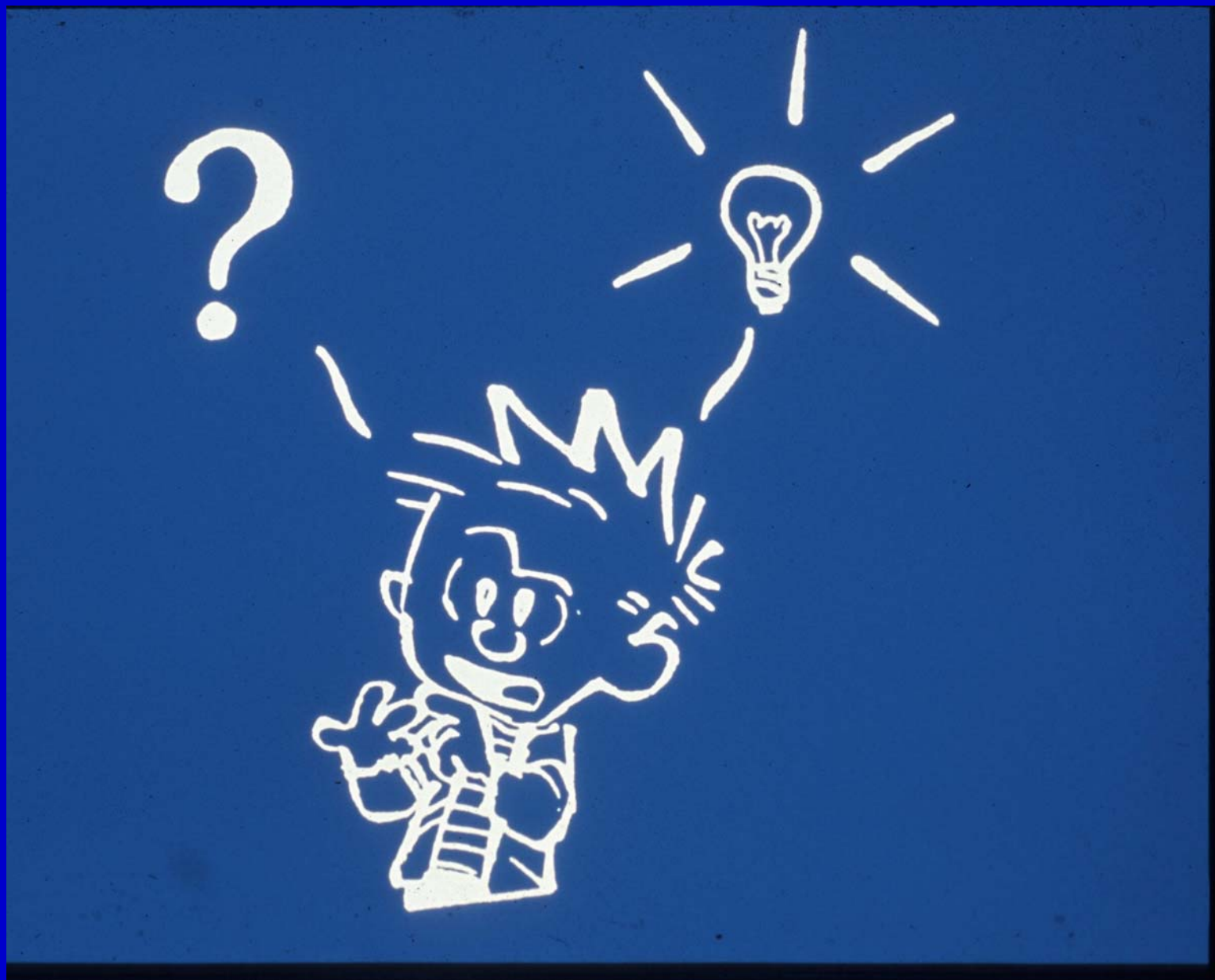
# REGRESSED FIELD CASE HISTORIES



# Key Elements of Soil Liquefaction Engineering







# Milestones in Geotechnical Earthquake Engineering

|      |                             |
|------|-----------------------------|
| 1964 | Great Alaskan Earthquake    |
| 1964 | Niigata Earthquake (Japan)  |
| 1971 | San Fernando Earthquake     |
| 1986 | Mexico City                 |
| 1989 | Loma Prieta Earthquake      |
| 1994 | Northridge Earthquake       |
| 1995 | Kobe Earthquake             |
| 1999 | Kocaeli (Turkey) Earthquake |
| 1999 | Chi-Chi (Taiwan) Earthquake |
| 2001 | Bhuj (India) Earthquake     |

# Key Attributes of “Important” Geotechnical Earthquake Investigations

1. Timely response.
  - Rapid (early) arrival of well-qualified “scouts”
  - Good communication
  - Adequate manpower and logistics for main study group(s)
2. Well-structured investigation teams.
  - “Experts” with good prior field experience
  - Less experienced team members (to learn....)
  - Very high levels of dedication, diligence, sacrifice, and teamwork
  - Good social consciousness and cultural sensitivity
3. “Local” involvement on the team.
4. Open sharing.
5. Diligent follow-on studies.



# Degirmendere

